February 13, 2020
Project No. M18-019
45 Lumber Road
Roslyn, New York 11577

## PROJECT SUMMARY

| Applicant: | 45 Lumber Rd. LLC |
| :--- | :--- |
| Zoning: | WMU - Waterfront Mixed-Use District |
| Previous Land Use: | Verizon Truck Depot |
| Proposed Land Use: | Residential Apartment Building <br> (33 units) |
| Location: | 45 Lumber Road |
| Tax Map: | Section 6, Block 53, Lot 1031 |
| Site Area: | 1.39 Acres (60,618 sf) |
| Existing Building Size: | 5,722 square feet (1 Story) |
| Proposed Building Size: | 89,714 square feet (4 Stories) |
| Required Parking: | 66 Parking Space |
| Proposed Parking: | 67 Parking Spaces |

Source:

Project summary based on information shown on the site plan prepared by:
Northcoast Civil Land Surveying \& Civil Engineering
23 Spring Street
Oyster Bay, New York 11771

## INTRODUCTION

Our office has conducted a traffic and parking study of the existing/proposed development of the subject property located at 45 Lumber Road, in the Village of Roslyn. The property is zoned WMU - Waterfront Mixed Use. The property is 60,618 square feet in size. The site is fully developed with Verizon Truck Depot (currently vacant) and associated parking.

The applicant is seeking to improve the site with a residential apartment building and associated parking. The building will be 4 stories and provide 33 two-bedroom apartment units.

In accordance with the Village of Roslyn zoning requirements, the proposed building requires 66 parking spaces. The proposed improvements will provide 67 parking spaces for the 33 apartment units.

## Public Transit

The area is served by the Long Island Railroad and Nassau Inter-County Express (NICE) bus service. The site is located within a mile of the Roslyn train station. Bus service surrounding the site includes the N23 and N27 routes.

## SITE ACCESS

The Nassau County Tax Map shows a $38.22^{\prime}$ width at the northern terminus of Lumber Road. The subject property extends approximately halfway along this end of the right-of-way. The other half of the right-of-way is bordered by the Independent Metal Strap property ( 34 Lumber Road). The Roslyn Hotel property ( 1221 Old Northern Boulevard) intersects at the corner of the right-of-way. A fourth property the Waterfront at Roslyn (55 Lumber Road) is located to the north of the subject site with no direct access to Lumber Road.

It appears that these properties share cross-access easements/agreements allowing access to and from Lumber Road. Access to the subject site will remain on private property. Access to Lumber Road will be located within the easement area.

## ROADWAY NETWORK

Lumber Road is a dead-end roadway starting at Old Northern Boulevard and terminating at the site frontage (as described above). Lumber Road provides one northbound and one southbound lane. Lumber Road provides the sole access to serval properties including a municipal parking lot. Lumber Road runs parallel to the Hempstead Harbor Creek.

## ACCIDENT ANALYSIS

Motor vehicle accident history reports pertaining to the study intersection were obtained from the New York State Department of Transportation. The reports document motor vehicle accidents that took place at the study intersections. The New York State Department of Transportation reports span a $36-$ month period beginning April 2015 and ending March 2018. A summary and detailed description of the accident history is provided in Tables No. 1 and 2, attached hereto.

Over the three-year period, a total of 8 accidents occurred at or in proximity to the intersection of Old Northern Boulevard and Lumber Road. On average, approximately 2.67 accidents occur per year in this area. During the same three-year period it is estimated that 12.2 million vehicles drove through this intersection. This equates to one accident for every 1.5 million vehicles that travel through the intersection.

The following provides an overview of the accident types:

| Accident Type | No. of Accidents | Percentage |
| :--- | :--- | :--- |
|  |  |  |
| Left Turn | 1 | $12.5 \%$ |
| Other | 1 | $12.5 \%$ |
| Overtaking | 1 | $12.5 \%$ |
| Rear End | 2 | $25.0 \%$ |
| Right Angle | 3 | $37.5 \%$ |
|  |  |  |
| Accident Severity | No. of Accidents | Percentage |
|  |  |  |
| Non-Reportable | 4 | $50.0 \%$ |
| Property Damage Only | 4 | $50.0 \%$ |

## ACCIDENT MITIGATION

No fatalities or serious injury were reported. All eight accidents were either non-reportable or involved property damage only. The low overall number of accidents over the three-year period does not appear to show a specific accident trend in the area surrounding the subject site.

The intersection does not experience a high number of serious motor vehicle accidents, as demonstrated by the State accident data. Observations of traffic flow at the intersection during peak hours does however indicate deficiencies which negatively impact vehicle movements to and from Lumber Road.

Our office prepared two alternative mitigation plans for the intersection of Old Northern Boulevard and Lumber Road. Any improvements at this intersection will require the review and approval of the Nassau County Department of Public Works, as Old Northern Boulevard is under the County's jurisdiction.

## Mitigation - Concept A

Concept A realigns the southernmost section of Lumber Road to intersection Old Northern Boulevard at a 90-degree angle.

Vehicle turning left from Old Northern Boulevard tend to crossover southbound lanes when entering onto Lumber Road. The re-alignment of the intersection is intended to reduce/eliminate this condition.

The design would eliminate 4 angled parking spaces on Lumber Road which are in close proximity to the intersection. One additional parking space would be removed on the south side of Old Northern Boulevard, just west of the entrance driveway to the municipal parking lot.

The intersection re-alignment of Lumber Road is accomplished, in part, by the use of bulbouts on the northeast and northwest corners. A third bulbout is shown on the southeast corner. The bulbouts are joined via pedestrian crosswalks. The Bulbout design allow southbound vehicles greater visibility to the west. The design also reduces travel distance for pedestrians crossing the intersection. The Old Northern Boulevard crossing is aligned with the Village Parking Lot on the south side of the roadway.

## Mitigation - Concept B

Concept B introduces a stiped island on the northwest corner of the intersection. The island is aligned with a proposed bulbout on the northeast corner of the intersection. The intent of this island is to define the westbound travel lane. The island also allows southbound motorist the ability to approach Old Northern Boulevard with greater visibility to the west within a defined southbound lane.

Vehicle turning left from Old Northern Boulevard tend to crossover southbound lanes when entering onto Lumber Road. The proposed pavement markings are intended to reduce/eliminate this condition.

The design would eliminate 4 angled parking spaces on Lumber Road which are in close proximity to the intersection. One additional parking space would be removed on the south side of Old Northern Boulevard, just west of the entrance driveway to the municipal parking lot.

Bulbouts are proposed on the northeast and southeast corners. The bulbouts are joined via pedestrian crosswalks. The design also reduces travel distance for pedestrians crossing Old Northern Boulevard. The Old Northern Boulevard crossing is aligned with the Village Parking Lot on the south side of the roadway.

## PARKING GENERATION

The parking generation of the site was calculated using the standard calculations compiled by the Institute of Transportation Engineers (ITE) in the 5th Edition Parking Generation, 2019. This is often referred to as the Parking Generation Manual and is considered the industry standard for traffic engineering studies.

The residential apartment units are estimated to generate approximately 45 parked vehicles. This peak parking demand will occur in the overnight hours. The estimated parking demand includes residents and guest.

According to the Census Bureau's Population Estimates Program 84.5\% of owner occupied households in the Village of Roslyn have 2 or fewer vehicles and $45.5 \%$ have no more than 1 vehicle. Vehicle ownership is a primary component of residential parking demand.

The proposed project supplies ample parking to accommodate the anticipated demand based on the ITE and Census data. The project meets and exceeds the parking requirements set forth in the Village Code.

## TRIP GENERATION

The subject site will generate a certain number of vehicle trips throughout the day. The volume of trips generated by the proposed development was calculated using the standard calculations compiled by the Institute of Transportation Engineers (ITE) in the $10^{\text {th }}$ Edition Trip Generation, 2017. This is often referred to as the Trip Generation Manual and is considered the industry standard for traffic engineering studies.

The trip generation of the proposed development was calculated using the ITE Land Use Code 221. The independent variable used in the calculation is the number of "number of units". This land use codes represent Mid-Rise Apartments.

The proposed site has the potential to generate a maximum of 15 peak hour trips (including entering and exiting trips). The proposed development has the potential to significantly decrease the number of vehicles generated by the subject site (if the site were to be re-occupied under existing conditions). The redevelopment will also decrease the potential amount of commercial truck traffic generated by the site. The trip generation calculations are provided in Table No. 3.

## TRIP DISTRIBUTION

Trips generated by the development of the subject site are distributed throughout the roadway network and assigned to the study intersections. The percent distribution is applied to the trip generation to establish the number of trips assigned to specific turning movements at each of the study intersections. One hundred percent of the trip generation is distributed and assigned to the site access.

A portion of the total trip generation is distributed and assigned to each of the other study intersections. The volume of trips assigned to each intersection is based on the percentage of vehicles that are anticipated to use these intersections while traveling to and from the site. The distribution is based on the existing traffic patterns on the roadway network.

## EXISTING Traffic Volumes

Turning movement counts were collected on Thursday, June $28^{\text {th }}$ and Saturday June $30^{\text {th }}$ of 2018. The counts were collected during the morning, afternoon and evening peak hours at the study intersections. Turning movement counts were collected during the typical peak times of the proposed site and surrounding roadway network.

Our office has previously collected turning movement counts at the intersection Old Northern Boulevard and Lumber Road. Turning movement counts collected in 2013 and 2015 are provide for reference. The turning movement volumes are shown on Table No. 4 through 11, attached hereto.

Turning movement counts were collected using Miovision Scout Video Collection Units and/or Electronic Jamar Traffic Data Collectors. The results of these traffic counts were analyzed to determine the distinct hour during each of the time periods surveyed when traffic experiences its highest level referred to as the "peak hour." The peak hour volume is used in our analysis to model the critical demand during each time period.

## AdJUSTED TRAFFIC VOLUME FLOW RATE

The Highway Capacity Analysis uses the adjusted flow rate based on the peak hour volume and the peak hour factor at each location. The peak hour volume is divided by the peak hour factor to produce the critical 15 -minute demand projected over the entire one-hour period. The results of this analysis provide the level of service experienced during the busiest 15 -minute period within the peak hour.

## AMBIENT TRAFFIC GROWTH

The volume of traffic using the roadway network changes each year based on population growth and development. An ambient growth rate is used to determine the future base traffic volumes. The ambient growth rate takes into account developments that will increase the volume of traffic at the study intersections prior to the completion of this project.

The existing traffic volumes at the study intersections were increased by a growth rate factor of $1.00 \%$ compounded yearly. This rate was applied based on conversations with the Nassau County Department of Public Works Traffic Engineering Department. The growth rate is applied to the existing volumes to generate the ambient no build traffic volumes.

For the purposes of this analysis, the future no build and build conditions are anticipated to occur within the next two years.

## Future No Build and Build Traffic

Our office met with the Roslyn Building Department to discuss project (other than the proposed application) that are currently under construction and/or projects that are planned to be completed within the next two years.

The Building Department identified Phase II of the Roslyn Landing project and an additional 5 studio units currently under construction within the 17 Lumber Road site. Trip generation and distribution studies were conducted at the sole entrance to the completed Phase I of the Roslyn Landing project.

Traffic attributed to these projects has been added to the ambient traffic volumes to estimate the future no build traffic volumes. These are the anticipated roadway volumes if no changes are made to the subject site. The future build traffic volumes include the trip generation of the proposed development.

## LEVEL OF SERVICE ANALYSIS:

The Level of Service Analysis prepared for the study intersections was conducted using Synchro. Syncro is a computer software program released by Trafficware, LLC. The software is based on the Highway Capacity Manual. The Highway Capacity Manual (HCM), developed by the Transportation Research Board (TRB), contains procedures for analyzing signalized and unsignalized intersections and is considered an appropriate analysis tool by most municipalities. Level of service ranges from A to F , based in part on the following criteria:

|  | Signalized Intersections <br> Average Delay (seconds/veh) | Stop Controlled Intersections <br> Average Delay (seconds/veh) |
| :--- | :---: | :---: |
| LOS A | $\leq 10$ | $\leq 10$ |
| LOS B | $>10-20$ | $>10-15$ |
| LOS C | $>20-35$ | $>15-25$ |
| LOS D | $>35-55$ | $>25-35$ |
| LOS E | $>55-80$ | $>35-50$ |
| LOS F | $>80$ | $>50$ |

Municipalities and agencies on Long Island do not have standardized policies or definitions of significant impact. There is also no industry wide standard for the definition of a significant impact. It is generally accepted that deterioration in levels of service (LOS) within the clearly acceptable range (LOS A through LOS C) is not considered significant. Information to support these statements is provided in the City Environmental Quality Review Technical Manual, March 2014 edition. The City Environmental Quality Review Technical Manual provides the following information relating to the determination of significant impact:

Section 411. Signalized Intersections: Determination of significant impacts for signalized intersections is summarized as follows: If a lane group under the With-Action (or "Build") condition is within LOS A, B or C, or marginally acceptable LOS D (average control delay less than or equal to 45.0 seconds/veh), the impact is not considered significant.

Section 412. Unsignalized Intersections: For unsignalized intersections the same criteria as for signalized intersections would apply. For the minor street to trigger a significant impact, 90 PCEs must be identified in the future With-Action conditions in any peak hour. (Please note, $a$ marginally acceptable LOS D for an unsignalized intersection would have an average control delay less than or equal to 30.0 seconds/veh).

## TRAFFIC IMPACTS

The study intersection will operate at acceptable levels of service upon completion of this project. The highway capacity analysis of the study intersection shows that the development of this property will have no significant impact to the level of service on the surrounding roadway network.

## Mitigation Measures

The highway capacity analysis indicates that off-site mitigation measures are not warranted at this time.

The proposed site is anticipated to generate approximately 1 trip every 4 minutes during hours of peak activity. Nassau County traffic signals typically complete between 40 and 60 cycles per hour (cycle length 60 to 90 seconds). The nearest traffic signals are located at the intersection of Old Northern Boulevard at E. Broadway (to the east) and W. Shore Road/Main Street (to the west). The volume of traffic generated by the site at either traffic signal will be less than one vehicle every 3 cycles on average.

## DEVELOPMENT INCENTIVE BONUSES

The Village Comprehensive Plan, July 1996, discusses vacant properties along the east side of Hempstead Harbor Creek describing them as "ripe for development". The properties what are now the Horizon at Roslyn (61 Bryant Avenue), Atria on Roslyn Harbor (100 Landing Road) and Roslyn Landing ( 1407 Old Northern Boulevard). These properties were formally industrial uses and are now residential.

A similar transition has recently occurred on the west side of Hempstead Harbor Creek. The former Lumber Yard located at 17 Lumber Road has been transformed into a residential property with retail stores on the ground floor. This project also included a promenade along the waterfront.

The applicant is seeking to convert the former Verizon Truck Depot into a residential development. As part of this project the applicant is reviewing potential improvements in order to receive development incentive bonuses, as outlined in the following sections of the Village Code:

## § 470-20 - WD-O Waterfront Development Overlay District C. - Development Incentive Bonuses

(6) The Board of Trustees, following a public hearing, may provide incentive bonuses in accordance with the schedule below in exchange for the applicant providing one or more of the following facilities or amenities:
(a) Public pedestrian and/or vehicular access to the waterfront and to water-dependent uses.
(d) Pedestrian linkages between contiguous uses or between the waterfront and downtown.
(f) Road improvements, on-street parking, pathway pavers, street trees, sidewalk extensions in parking lanes to slow vehicular traffic, and other elements which make roads more pedestrian friendly.
(j) Provision of road and/or traffic signalization and control improvements upon those public streets which may be impacted by the project or development.

## ${ }^{1}$ CONCLUSIONS:

The Village's Comprehensive Plan was prepared over 20 years ago. In 2016, the Village prepared a Village Parking and Traffic Study. These studies outline traffic issues along Old Northern Boulevard which have not yet been resolved.

Our analysis indicates that the site provides ample parking to accommodate the anticipated peak demand. The volume of traffic generated by the proposed development is not anticipated to impact the level of service of the surrounding roadway network.

Although not warranted by the trip generation of the subject site; NCDPW ROW Plans Concept A and B are provided for the Village's review and consideration. As stated, any improvements at the intersection of Old Northern Boulevard and Lumber Road will require the review and approval of the Nassau County Department of Public Works.

In our professional opinion, the granting of this application will not have an adverse impact on the surrounding roadway network. If you have any questions or require additional information please feel free to contact our office.


Sean P. Mulryan, P.E.
President

[^0]| Location: | Old Northern Blvd at Lumber Rd |
| :--- | :--- |
| Period Covered: | $\underline{04 / 2015-3 / 2018}$ |
| Date: | $\underline{07-2018}$ |

$\qquad$
County: Nassau


| TE 213 (9/79) DETAILS OF ACCIDENT HISTORY FOR LOCATION Table No. 2 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| M18-019 |  |  |  |  | OLD NORTHERN BOULEVARD |  |  |  |  |  |  |  |  | NASSAU COUNTY VILLAGE OF ROSLYN 07-2018 |
|  |  |  |  |  | AT INTERSECTION WITH LUMBER ROAD |  |  |  |  |  |  |  |  |  |
| NO. OF MONTHS |  |  | LIGHT CONDITIONS (LC) |  |  |  |  | ROADWAY CHARACTER (RC) |  |  |  | ROADWAY SURFACE CONDITION (RSC) |  |  |
|  |  |  | 1. Daylight <br> 2. Dawn |  |  |  |  | 1. Straight \& Level |  |  |  | 1. Dry |  | 1. Clear |
|  |  |  |  |  |  |  |  | 2. Straight \& Grade <br> 3. Straight at Hillcrest |  |  |  | 2. Wet <br> 3. Muddy |  | 2. Cloudy |
|  |  |  | 4. Dark Road Lighted |  |  |  |  |  |  |  |  | 3. Rain |  |
| Begin Date: 04-2015 |  |  |  |  |  |  |  | 4. Curve \& Level |  |  |  |  |  | 4. Snow/lce |  | 4. Snow |
| End Date: 03-2018 |  |  | 5. Dark Road Unlighted |  |  |  |  | 5. Curve \& Grade <br> 6. Curve at Hillcrest |  |  |  | 5. Slush <br> 10. Other |  | 5. Sleet/Hail/Freezing Rain 6. Fog/Smog/Smoke |
|  |  |  | 10. Other |  |  |  |  |  |  |  |
| NO | CASE | DATE |  |  |  |  |  | TIME | $\begin{aligned} & \text { \# OF } \\ & \text { VEH } \end{aligned}$ | SEV | LC | RC | RSC | WEA | CONTRIB <br> FACTORS | $\begin{aligned} & \text { REF } \\ & \text { MKR } \end{aligned}$ | ACC TYPE | DES | IPTION |
| 1 | 36414449 | 10/7/2016 | 11:40 | 2 | NR | 1 | 1 | 1 | 1 | 07, YY |  | LEFT TURN (AGAINST OTHER CAR) | VEHICLE 1 AND VEHICLE 2 WERE IN REMOVED FROM SCENE BY OPERA | COLLISION. BOTH VEHICLES RS. |
| 2 | 37160445 | 2/26/2018 | 16:01 | 2 | PDO | 1 | 1 | 1 | 2 | 03, YY |  | RIGHT ANGLE | VEHICLES 1 AND 2 WERE IN COLLIS OPERATORS. | N. BOTH VEHICLES REMOVED BY |
| 3 | 36777817 | 6/23/2017 | 16:30 | 2 | NR | 1 | 1 | 1 | 1 | 07, YY |  | RIGHT ANGLE | DRIVER VEHILCLE 2 STATES WHILE NORTHERN BLVD SHE WAS IN A CO ENTERING THE ROADWAY FROM T REMOVED BY OPERATORS. | RAVELING STRAIGHT ON OLD ISION WITH VEHICLE 1 WHO WAS PARKING LOT. BOTH VEHICLES |
| 4 | 35909596 | 10/5/2015 | 15:45 | 2 | NR | 1 | 2 | 1 | 1 | 09, YY |  | REAR END | VEH. 1 AND VEH. 2 WERE IN A COLL REMOVED FROM SCENE BY OPERA BEHIND VEH. 2 WHEN VEH. 2 STOP HE COULD NOT STOP IN TIME AND STATES HE WAS STOPPED IN TRAF VEH. 2 ATTEMPTING TO MAKE A LE | ON. BOTH VEHICLES WERE RS. OP. VEH. 1 STATES HE WAS D ABRUPTLY IN THE ROADWAY AND H. 1 STRUCK VEH. 2 OP. VEH. 2 DUE TO A VEHICLE IN FRONT OF TURN WHEN VEH. 1 STRUCK VEH. 2 |
| 5 | 36921407 | 10/6/2017 | 10:08 | 2 | NR | 1 | 1 | 1 | 2 | 09, YY |  | REAR END | VEHICLES 1 AND 2 WERE IN COLLIS SCENE. | N. BOTH VEHICLES LEFT PARKED AT |
| 6 | 36157572 | 4/2/2016 | 20:20 | 3 | PDO | 4 | 2 | 1 | 2 | 13, 19, YY |  | OTHER | MV \#1 WAS IN A COLLISION MV\#2. MV\#3. DRIVER OF MV\#1 STATED HE COLLIDED WITH MV\#2. DRIVER OF SYSTEMS(A VALET SERVICE), 284 T AND WAS ATTEMPTING TO PARK T | 2 WAS THEN IN A COLLISION WITH ADE THE TURN GOING TOO FAST AND \#1 IS AN EMPLOYEE OF PARKING STREET VALLEY STREAM NY 11581, VEHICLE. |
| 7 | 36628453 | 3/3/2017 | 17:33 | 2 | PDO | 1 | 1 | 1 | 2 | 04, 20, YY |  | OVERTAKING | VEHICLE 1 STRUCK VEHICLE 2. BO OPERATORS. ROSLYN VILLAGE NOT THOUGHT THAT SHE WAS IN REVE STRUCK VEHICLE 2 , JUMPED THE OVER A BUST STREET AND JUMPED STRONGLY RECOMMENDED. | VEHICLES REMOVED BY IED OF DAMAGE. DRIVER 1 <br> , VEHICLE WAS IN DRIVE AND <br> , STRUCK LIGHT POLE , CROSSED NOTHER CURB. DRIVER REVIEW IS |


|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| M18-019 |  |  |  |  | OLD NORTHERN BOULEVARD |  |  |  |  |  |  |  |  | NASSAU COUNTY <br> VILLAGE OF ROSLYN 07-2018 |
|  |  |  |  |  | AT INTERSECTION WITH LUMBER ROAD |  |  |  |  |  |  |  |  |  |
| NO. OF MONTHS |  |  | LIGHT CONDITIONS (LC) |  |  |  |  | ROADWAY CHARACTER (RC) |  |  |  | ROADWAY SURFACE CONDITION (RSC) |  | WEATHER (WEA) |
|  |  |  | 1. Daylight <br> 2. Dawn |  |  |  |  | 1. Straight \& Level <br> 2. Straight \& Grade |  |  |  |  |  | 1. Clear 2. Cloudy |
| Begin Date: 04-2015 |  |  | 3. Dusk <br> 4. Dark Road Lighted |  |  |  |  | 3. Straight at Hillcrest <br> 4. Curve \& Level |  |  |  |  |  | 2. Cloudy 3. Rain |
|  |  |  | 4. Snov |  | 4. Snow |  |  |  |  |  |
| End Date: 03-2018 |  |  |  |  |  |  |  | 5. Dark Road Unlighted |  |  |  |  | 5. Curve \& Grade <br> 6. Curve at Hillcrest |  |  |  | 5. Slush 10. Other |  | 5. Sleet/Hail/Freezing Rain |
|  |  |  | 6. Fog/Smog/Smoke |  |  |  |  |  |  |  |  |  |  |  |  |
| NO | CASE | DATE | TIME | $\begin{aligned} & \text { \# OF } \\ & \text { VEH } \end{aligned}$ | SEV | LC | RC | RSC | WEA | CONTRIB <br> FACTORS | $\begin{gathered} \text { REF } \\ \text { MKR } \end{gathered}$ | ACC TYPE | DESC | IPTION |  |  |
| 8 | 36259438 | 6/17/2016 | 14:00 | 2 | PDO | 1 | 1 | 1 | 1 | 07, 18, YY |  | RIGHT ANGLE | VEHICLES 1 AND 2 WERE IN COLLIS OPERATORS. DRIVER 1 WAS NOT required info. | N. BOTH VEHICLES REMOVED by SCENE BUT DID LEAVE ALL |  |  |




(2) sum

## Trip Generation Calculations

## Proposed Development

Land Use Code:
Land Use Description: Independent Variable:
Variable:
Source:

|  | Directional <br> Distribution | Rate | Standard <br> Deviation | Adjustment <br> Factor | Driveway <br> Volume |
| :--- | :---: | :---: | :---: | :---: | :---: |
| 7-9 AM Peak Hour Enter | $26 \%$ | 0.09 | 0.00 | 1.00 | 3 |
| 7-9 AM Peak Hour Exit | $\underline{74 \%}$ | $\underline{0.27}$ | 0.00 | 1.00 | $\underline{9}$ |
| 7-9 AM Peak Hour Total | $100 \%$ | 0.36 | 0.19 | 1.00 | 12 |
| AM Peak Hour Enter | $27 \%$ | 0.09 | 0.00 | 1.00 | $\underline{3}$ |
| AM Peak Hour Exit | $\underline{73 \%}$ | $\underline{0.23}$ | 0.00 | 1.00 | $\underline{8}$ |
| AM Peak Hour Total | $100 \%$ | 0.32 | 0.17 | 1.00 | 11 |
| PM Peak Hour Enter | $60 \%$ | 0.25 | 0.00 | 1.00 | $\underline{8}$ |
| PM Peak Hour Exit | $\underline{40 \%}$ | $\underline{0.16}$ | 0.00 | 1.00 | $\underline{5}$ |
| PM Peak Hour Total | $100 \%$ | 0.41 | 0.22 | 1.00 | 14 |
| 4-6 PM Peak Hour Enter | $61 \%$ | 0.27 | 0.00 | 1.00 | 9 |
| 4-6 PM Peak Hour Exit | $\underline{39 \%}$ | $\underline{0.17}$ | 0.00 | 1.00 | $\underline{6}$ |
| 4-6 PM Peak Hour Total | $100 \%$ | 0.44 | 0.19 | 1.00 | 15 |
| Saturday Peak Hour Enter | $49 \%$ | 0.22 | 0.00 | 1.00 | 7 |
| Saturday Peak Hour Exit | $\underline{51 \%}$ | $\underline{0.22}$ | 0.00 | 1.00 | $\underline{7}$ |
| Saturday Peak Hour Total | $100 \%$ | 0.44 | 0.08 | 1.00 | 15 |


| Mulryan Engineering, P.C. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Table No. 4 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hamlet: <br> Project No. |  | Village of Roslyn M18-019 |  | Turning Movement Counts Thursday, June 28, 2018 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Lumber Road atOld Northern Boulevard |  |  | U-Turn | Right | Southbound Through | Left | U-Turn | Right | Westbound Through | Left | U-Turn | Right | Northbound Through | Left | U-Turn | Right | Eastbound Through | Left | Vehicle Total |
| AM Turning Movement Counts |  | 7:00 AM | 0 | 6 | 0 | 3 | 0 | 2 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 62 | 11 | 128 |
|  |  | 7:15 AM | 0 | 4 | 0 | 3 | 0 | 0 | 75 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 69 | 5 | 156 |
|  |  | 7:30 AM | 0 | 3 | 0 | 1 | 0 | 1 | 103 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 | 9 | 177 |
|  |  | 7:45 AM | 0 | 4 | 0 | 2 | 0 | 3 | 89 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 88 | 13 | 199 |
|  |  | 8:00 AM | 0 | 4 | 0 | 5 | 0 | 3 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 60 | 4 | 160 |
|  |  | 8:15 AM | 0 | 5 | 0 | 4 | 0 | 4 | 106 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 73 | 10 | 202 |
|  |  | 8:30 AM | 0 | 5 | 0 | 4 | 0 | 8 | 112 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 86 | 8 | 223 |
|  |  | 8:45 AM | 0 | 9 | 0 | 1 | 0 | 8 | 125 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 80 | 16 | 239 |
| 7:00 AM | to | 8:00 AM | 0 | 17 | 0 | 9 | 0 | 6 | 311 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 279 | 38 | 660 |
| 7:15 AM | to | 8:15 AM | 0 | 15 | 0 | 11 | 0 | 7 | 351 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 277 | 31 | 692 |
| 7:30 AM | to | 8:30 AM | 0 | 16 | 0 | 12 | 0 | 11 | 382 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 281 | 36 | 738 |
| 7:45 AM | to | 8:45 AM | 0 | 18 | 0 | 15 | 0 | 18 | 391 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 307 | 35 | 784 |
| 8:00 AM | to | 9:00 AM | 0 | 23 | 0 | 14 | 0 | 23 | 427 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 299 | 38 | 824 |
| Midday Turning Movement Counts |  | 12:00 PM | 0 | 19 | 0 | 8 | 0 | 6 | 86 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 147 | 18 | 284 |
|  |  | 12:15 PM | 0 | 20 | 0 | 10 | 0 | 10 | 85 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 154 | 20 | 299 |
|  |  | 12:30 PM | 0 | 26 | 0 | 6 | 0 | 6 | 92 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 165 | 26 | 321 |
|  |  | 12:45 PM | 0 | 15 | 0 | 12 | 0 | 3 | 112 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 164 | 18 | 324 |
|  |  | 1:00 PM | 0 | 18 | 0 | 13 | 0 | 13 | 116 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 138 | 12 | 310 |
|  |  | 1:15 PM | 0 | 16 | 0 | 7 | 0 | 8 | 109 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 127 | 12 | 279 |
|  |  | 1:30 PM | 0 | 13 | 0 | 12 | 0 | 6 | 87 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 110 | 10 | 238 |
|  |  | 1:45 PM | 0 | 16 | 0 | 7 | 0 | 6 | 95 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 147 | 10 | 281 |
| 12:00 PM | to | 1:00 PM | 0 | 80 | 0 | 36 | 0 | 25 | 375 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 630 | 82 | 1228 |
| 12:15 PM | to | 1:15 PM | 0 | 79 | 0 | 41 | 0 | 32 | 405 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 621 | 76 | 1254 |
| 12:30 PM | to | 1:30 PM | 0 | 75 | 0 | 38 | 0 | 30 | 429 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 594 | 68 | 1234 |
| 12:45 PM | to | 1:45 PM | 0 | 62 | 0 | 44 | 0 | 30 | 424 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 539 | 52 | 1151 |
| 1:00 PM | to | 2:00 PM | 0 | 63 | 0 | 39 | 0 | 33 | 407 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 522 | 44 | 1108 |
| PM Turning Movement Counts |  | 3:00 PM | 0 | 15 | 0 | 8 | 0 | 6 | 79 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 142 | 7 | 257 |
|  |  | 3:15 PM | 0 | 8 | 0 | 6 | 0 | 5 | 88 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 131 | 9 | 247 |
|  |  | 3:30 PM | 0 | 14 | 0 | 8 | 0 | 5 | 67 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 138 | 7 | 239 |
|  |  | 3:45 PM | 0 | 19 | 0 | 7 | 0 | 6 | 70 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 149 | 11 | 262 |
|  |  | 4:00 PM | 0 | 19 | 0 | 5 | 0 | 4 | 92 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 167 | 17 | 304 |
|  |  | 4:15 PM | 0 | 12 | 0 | 8 | 0 | 7 | 86 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 157 | 12 | 282 |
|  |  | 4:30 PM | 0 | 11 | 0 | 6 | 0 | 4 | 69 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 157 | 17 | 264 |
|  |  | 4:45 PM | 0 | 14 | 0 | 2 | 0 | 0 | 82 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 13 | 264 |
|  |  | 5:00 PM | 0 | 21 | 0 | 4 | 0 | 5 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 154 | 11 | 260 |
|  |  | 5:15 PM | 0 | 16 | 0 | 11 | 0 | 6 | 74 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 129 | 19 | 255 |
|  |  | 5:30 PM | 0 | 12 | 0 | 12 | 0 | 4 | 63 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 127 | 9 | 227 |
|  |  | 5:45 PM | 0 | 12 | 0 | 3 | 0 | 1 | 70 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 107 | 17 | 210 |
|  |  | 6:00 PM | 0 | 17 | 0 | 17 | 0 | 7 | 76 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 137 | 6 | 260 |
|  |  | 6:15 PM | 0 | 9 | 0 | 6 | 0 | 0 | 67 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 118 | 8 | 208 |
|  |  | 6:30 PM | 0 | 10 | 0 | 5 | 0 | 6 | 66 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 129 | 7 | 223 |
|  |  | 6:45 PM | 0 | 10 | 0 | 4 | 0 | 6 | 76 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 135 | 14 | 245 |
|  |  | 7:00 PM | 0 | 8 | 0 | 2 | 0 | 1 | 66 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 116 | 17 | 210 |
|  |  | 7:15 PM | 0 | 9 | 0 | 5 | 0 | 5 | 51 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 127 | 12 | 209 |
|  |  | 7:30 PM | 0 | 13 | 0 | 7 | 0 | 4 | 42 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 102 | 20 | 188 |
|  |  | 7:45 PM | 0 | 10 | 0 | 5 | 0 | 5 | 53 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 115 | 25 | 213 |
| 3:00 PM | to | 4:00 PM | 0 | 56 | 0 | 29 | 0 | 22 | 304 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 560 | 34 | 1005 |
| 3:15 PM | to | 4:15 PM | 0 | 60 | 0 | 26 | 0 | 20 | 317 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 585 | 44 | 1052 |
| 3:30 PM | to | 4:30 PM | 0 | 64 | 0 | 28 | 0 | 22 | 315 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 611 | 47 | 1087 |
| 3:45 PM | to | 4:45 PM | 0 | 61 | 0 | 26 | 0 | 21 | 317 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 630 | 57 | 1112 |
| 4:00 PM | to | 5:00 PM | 0 | 56 | 0 | 21 | 0 | 15 | 329 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 634 | 59 | 1114 |
| 4:15 PM | to | 5:15 PM | 0 | 58 | 0 | 20 | 0 | 16 | 302 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 621 | 53 | 1070 |
| 4:30 PM | to | 5:30 PM | 0 | 62 | 0 | 23 | 0 | 15 | 290 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 593 | 60 | 1043 |
| 4:45 PM | to | 5:45 PM | 0 | 63 | 0 | 29 | 0 | 15 | 284 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 563 | 52 | 1006 |
| 5:00 PM | to | 6:00 PM | 0 | 61 | 0 | 30 | 0 | 16 | 272 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 517 | 56 | 952 |
| 5:15 PM | to | 6:15 PM | 0 | 57 | 0 | 43 | 0 | 18 | 283 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 500 | 51 | 952 |
| 5:30 PM | to | 6:30 PM | 0 | 50 | 0 | 38 | 0 | 12 | 276 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 489 | 40 | 905 |
| 5:45 PM | to | 6:45 PM | 0 | 48 | 0 | 31 | 0 | 14 | 279 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 491 | 38 | 901 |
| 6:00 PM | to | 7:00 PM | 0 | 46 | 0 | 32 | 0 | 19 | 285 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 519 | 35 | 936 |
| 6:15 PM | to | 7:15 PM | 0 | 37 | 0 | 17 | 0 | 13 | 275 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 498 | 46 | 886 |
| 6:30 PM | to | 7:30 PM | 0 | 37 | 0 | 16 | 0 | 18 | 259 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 507 | 50 | 887 |
| 6:45 PM | to | 7:45 PM | 0 | 40 | 0 | 18 | 0 | 16 | 235 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 480 | 63 | 852 |
| 7:00 PM | to | 8:00 PM | 0 | 40 | 0 | 19 | 0 | 15 | 212 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 460 | 74 | 820 |
| Peak Hour | PHF | Start Time |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM | 0.862 | 8:00 AM | 0 | 23 | 0 | 14 | 0 | 23 | 427 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 299 | 38 | 824 |
| Midday | 0.968 | 12:15 PM | 0 | 79 | 0 | 41 | 0 | 32 | 405 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 621 | 76 | 1254 |
| PM | 0.916 | 4:00 PM | 0 | 56 | 0 | 21 | 0 | 15 | 329 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 634 | 59 | 1114 |





| Mulryan Engineering, P.C. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Table No. 8 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hamlet: <br> Project No. |  | Village of Roslyn M18-019 |  | Turning Movement Counts Thursday, June 28, 2018 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| Mill Creek South at Old Northern Boulevard |  |  |  | Southbound |  |  | Westbound |  |  |  | Northbound |  |  |  | U-Turn | Eastbound |  |  | $\begin{array}{\|c\|} \hline \text { Vehicle } \\ \text { Total } \\ \hline \end{array}$ |
|  |  |  | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left |  | Right | Through | Left |  |
| AM Turning Movement Counts |  | 7:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 44 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 55 | 1 | 100 |
|  |  | 7:15 AM | 0 | 1 | 0 | 0 | 0 | 0 | 72 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 69 | 0 | 142 |
|  |  | 7:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 98 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 63 | 0 | 161 |
|  |  | 7:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 91 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 85 | 2 | 178 |
|  |  | 8:00 AM | 0 | 0 | 0 | 0 | 0 | 0 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 57 | 1 | 142 |
|  |  | 8:15 AM | 0 | 1 | 0 | 2 | 0 | 3 | 110 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 76 | 2 | 194 |
|  |  | 8:30 AM | 0 | 2 | 0 | 0 | 0 | 1 | 119 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 91 | 2 | 215 |
|  |  | 8:45 AM | 0 | 1 | 0 | 1 | 0 | 0 | 127 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 80 | 1 | 210 |
| 7:00 AM | to | 8:00 AM | 0 | 1 | 0 | 0 | 0 | 0 | 305 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 272 | 3 | 581 |
| 7:15 AM | to | 8:15 AM | 0 | 1 | 0 | 0 | 0 | 0 | 345 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 274 | 3 | 623 |
| 7:30 AM | to | 8:30 AM | 0 | 1 | 0 | 2 | 0 | 3 | 383 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 281 | 5 | 675 |
| 7:45 AM | to | 8:45 AM | 0 | 3 | 0 | 2 | 0 | 4 | 404 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 309 | 7 | 729 |
| 8:00 AM | to | 9:00 AM | 0 | 4 | 0 | 3 | 0 | 4 | 440 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 304 | 6 | 761 |
| Midday Turning Movement Counts |  | 12:00 PM | 0 | 7 | 0 | 3 | 0 | 3 | 85 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 130 | 6 | 234 |
|  |  | 12:15 PM | 0 | 5 | 0 | 2 | 0 | 2 | 78 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 151 | 1 | 239 |
|  |  | 12:30 PM | 0 | 4 | 0 | 3 | 0 | 0 | 93 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 152 | 5 | 257 |
|  |  | 12:45 PM | 0 | 5 | 0 | 1 | 0 | 0 | 110 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 158 | 1 | 275 |
|  |  | 1:00 PM | 0 | 3 | 0 | 0 | 0 | 0 | 122 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 145 | 4 | 274 |
|  |  | 1:15 PM | 0 | 3 | 0 | 3 | 0 | 2 | 105 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 138 | 0 | 251 |
|  |  | 1:30 PM | 0 | 0 | 0 | 1 | 0 | 1 | 91 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 123 | 1 | 217 |
|  |  | 1:45 PM | 0 | 5 | 0 | 1 | 0 | 2 | 93 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 146 | 4 | 251 |
| 12:00 PM | to | 1:00 PM | 0 | 21 | 0 | 9 | 0 | 5 | 366 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 591 | 13 | 1005 |
| 12:15 PM | to | 1:15 PM | 0 | 17 | 0 | 6 | 0 | 2 | 403 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 606 | 11 | 1045 |
| 12:30 PM | to | 1:30 PM | 0 | 15 | 0 | 7 | 0 | 2 | 430 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 593 | 10 | 1057 |
| 12:45 PM | to | 1:45 PM | 0 | 11 | 0 | 5 | 0 | 3 | 428 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 564 | 6 | 1017 |
| 1:00 PM | to | 2:00 PM | 0 | 11 | 0 | 5 | 0 | 5 | 411 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 552 | 9 | 993 |
| PM Turning Movement Counts |  | 3:00 PM | 0 | 1 | 0 | 1 | 0 | 0 | 80 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 150 | 4 | 236 |
|  |  | 3:15 PM | 0 | 2 | 0 | 3 | 0 | 4 | 86 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 127 | 5 | 227 |
|  |  | 3:30 PM | 0 | 2 | 0 | 2 | 0 | 3 | 70 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 142 | 2 | 221 |
|  |  | 3:45 PM | 0 | 1 | 0 | 4 | 0 | 1 | 77 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 6 | 242 |
|  |  | 4:00 PM | 0 | 5 | 0 | 4 | 0 | 2 | 85 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 161 | 0 | 257 |
|  |  | 4:15 PM | 0 | 0 | 0 | 3 | 0 | 1 | 94 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 179 | 0 | 277 |
|  |  | 4:30 PM | 0 | 2 | 0 | 1 | 0 | 0 | 67 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 157 | 3 | 230 |
|  |  | 4:45 PM | 0 | 1 | 0 | 0 | 0 | 0 | 84 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 161 | 2 | 248 |
|  |  | 5:00 PM | 0 | 1 | 0 | 2 | 0 | 3 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 148 | 1 | 220 |
|  |  | 5:15 PM | 0 | 0 | 0 | 1 | 0 | 1 | 78 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 143 | 2 | 225 |
|  |  | 5:30 PM | 0 | 4 | 0 | 1 | 0 | 1 | 62 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 136 | 1 | 205 |
|  |  | 5:45 PM | 0 | 1 | 0 | 1 | 0 | 2 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 110 | 0 | 182 |
|  |  | 6:00 PM | 0 | 2 | 0 | 4 | 0 | 0 | 78 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 153 | 2 | 239 |
|  |  | 6:15 PM | 0 | 0 | 0 | 1 | 0 | 0 | 67 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 126 | 0 | 194 |
|  |  | 6:30 PM | 0 | 2 | 0 | 1 | 0 | 0 | 68 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 133 | 0 | 204 |
|  |  | 6:45 PM | 0 | 1 | 0 | 0 | 0 | 1 | 79 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 133 | 1 | 215 |
|  |  | 7:00 PM | 0 | 1 | 0 | 0 | 0 | 0 | 60 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 112 | 0 | 173 |
|  |  | 7:15 PM | 0 | 1 | 0 | 0 | 0 | 1 | 65 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 131 | 1 | 199 |
|  |  | 7:30 PM | 0 | 0 | 0 | 3 | 0 | 1 | 52 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 98 | 1 | 155 |
|  |  | 7:45 PM | 0 | 1 | 0 | 0 | 0 | 1 | 56 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 109 | 3 | 170 |
| 3:00 PM | to | 4:00 PM | 0 | 6 | 0 | 10 | 0 | 8 | 313 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 572 | 17 | 926 |
| 3:15 PM | to | 4:15 PM | 0 | 10 | 0 | 13 | 0 | 10 | 318 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 583 | 13 | 947 |
| 3:30 PM | to | 4:30 PM | 0 | 8 | 0 | 13 | 0 | 7 | 326 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 635 | 8 | 997 |
| 3:45 PM | to | 4:45 PM | 0 | 8 | 0 | 12 | 0 | 4 | 323 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 650 | 9 | 1006 |
| 4:00 PM | to | 5:00 PM | 0 | 8 | 0 | 8 | 0 | 3 | 330 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 658 | 5 | 1012 |
| 4:15 PM | to | 5:15 PM | 0 | 4 | 0 | 6 | 0 | 4 | 310 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 645 | 6 | 975 |
| 4:30 PM | to | 5:30 PM | 0 | 4 | 0 | 4 | 0 | 4 | 294 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 609 | 8 | 923 |
| 4:45 PM | to | 5:45 PM | 0 | 6 | 0 | 4 | 0 | 5 | 289 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 588 | 6 | 898 |
| 5:00 PM | to | 6:00 PM | 0 | 6 | 0 | 5 | 0 | 7 | 273 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 537 | 4 | 832 |
| 5:15 PM | to | 6:15 PM | 0 | 7 | 0 | 7 | 0 | 4 | 286 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 542 | 5 | 851 |
| 5:30 PM | to | 6:30 PM | 0 | 7 | 0 | 7 | 0 | 3 | 275 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 525 | 3 | 820 |
| 5:45 PM | to | 6:45 PM | 0 | 5 | 0 | 7 | 0 | 2 | 281 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 522 | 2 | 819 |
| 6:00 PM | to | 7:00 PM | 0 | 5 | 0 | 6 | 0 | 1 | 292 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 545 | 3 | 852 |
| 6:15 PM | to | 7:15 PM | 0 | 4 | 0 | 2 | 0 | 1 | 274 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 504 | 1 | 786 |
| 6:30 PM | to | 7:30 PM | 0 | 5 | 0 | 1 | 0 | 2 | 272 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 509 | 2 | 791 |
| 6:45 PM | to | 7:45 PM | 0 | 3 | 0 | 3 | 0 | 3 | 256 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 474 | 3 | 742 |
| 7:00 PM | to | 8:00 PM | 0 | 3 | 0 | 3 | 0 | 3 | 233 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 450 | 5 | 697 |
| Peak Hour | PHF | Start Time |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM | 0.885 | 8:00 AM | 0 | 4 | 0 | 3 | 0 | 4 | 440 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 304 | 6 | 761 |
| Midday | 0.961 | 12:30 PM | 0 | 15 | 0 | 7 | 0 | 2 | 430 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 593 | 10 | 1057 |
| PM | 0.913 | 4:00 PM | 0 | 8 | 0 | 8 | 0 | 3 | 330 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 658 | 5 | 1012 |





| Hamlet: | Village of Roslyn |
| :--- | :--- |
| Project No. | M18-019 |

## Trip Generation Calculations

| Proposed Development |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Land Use Description: <br> Independent Variable: <br> Variable: <br> Source: | Roslyn Landing Phase I |  |  |  |  |
|  | Number of Units |  |  |  |  |
|  | 28 |  |  |  |  |
|  | * Turing Movement Counts |  |  |  |  |
|  | Directional Distribution | Rate | Standard <br> Deviation | Adjustment Factor | Driveway Volume |
| 7-9 AM Peak Hour Enter | 69\% | 0.32 | 0.00 | 1.00 | 9 |
| 7-9 AM Peak Hour Exit | 31\% | 0.14 | 0.00 | 1.00 | 4 |
| 7-9 AM Peak Hour Total | 100\% | 0.46 | 0.00 | 1.00 | 13 |
| 12-2 PM Peak Hour Enter | 38\% | 0.21 | 0.00 | 1.00 | 6 |
| 12-2 PM Peak Hour Exit | 63\% | $\underline{0.36}$ | 0.00 | 1.00 | $\underline{10}$ |
| 12-2 PM Peak Hour Total | 100\% | 0.57 | 0.00 | 1.00 | 16 |
| 3-8 PM Peak Hour Enter | 29\% | 0.07 | 0.00 | 1.00 | 2 |
| 3-8 PM Peak Hour Exit | 71\% | $\underline{0.18}$ | 0.00 | 1.00 | 5 |
| 3-8 PM Peak Hour Total | 100\% | 0.25 | 0.00 | 1.00 | 7 |
| Saturday Peak Hour Enter | 22\% | 0.07 | 0.00 | 1.00 | 2 |
| Saturday Peak Hour Exit | 78\% | $\underline{0.25}$ | 0.00 | 1.00 | 7 |
| Saturday Peak Hour Total | 100\% | 0.32 | 0.00 | 1.00 | 9 |

* Turning movements were collected at the intersection of Old Northern Boulverad and Mill Creek South. This intersection also provides access to the The Junior League of Long Island Thrift Shop located at 1395 Old Northern Boulevard. The turning movement counts also include vehicles that enter the roadway and make u-turns. The trip generation numbers represented vehilces entering and exiting the residential development during the peak hour of the intersection.


## Estimated Trip Generation of Phase II (50 units)

| 7-9 AM Peak Hour Enter | $69 \%$ | 0.32 | 0.00 | 1.00 | 16 |
| :--- | :--- | :--- | :--- | :--- | :---: |
| 7-9 AM Peak Hour Exit | $\underline{31 \%}$ | $\underline{0.14}$ | 0.00 | 1.00 | $\underline{7}$ |
| 7-9 AM Peak Hour Total | $100 \%$ | 0.46 | 0.00 | 1.00 | 23 |
| 12-2 PM Peak Hour Enter | $38 \%$ | 0.21 | 0.00 | 1.00 | 11 |
| 12-2 PM Peak Hour Exit | $\underline{63 \%}$ | $\underline{0.36}$ | 0.00 | 1.00 | $\underline{18}$ |
| 12-2 PM Peak Hour Total | $100 \%$ | 0.57 | 0.00 | 1.00 | 29 |
|  |  |  |  |  |  |
| 3-8 PM Peak Hour Enter | $29 \%$ | 0.07 | 0.00 | 1.00 | 4 |
| 3-8 PM Peak Hour Exit | $\underline{71 \%}$ | $\underline{0.18}$ | 0.00 | 1.00 | $\underline{9}$ |
| 3-8 PM Peak Hour Total | $100 \%$ | 0.25 | 0.00 | 1.00 | 13 |
| Saturday Peak Hour Enter | $22 \%$ | 0.07 | 0.00 | 1.00 | 4 |
| Saturday Peak Hour Exit | $\underline{78 \%}$ | $\underline{0.25}$ | 0.00 | 1.00 | $\underline{13}$ |
| Saturday Peak Hour Total | $100 \%$ | 0.32 | 0.00 | 1.00 | 16 |


| Mulryan Engineerin | P.C. |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | able | O. 13 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Hamlet: Village of Rosl <br> Project No. M18-019 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | $17 \text { Lum }$ Trip Ge | ber Road neration $\mathbf{D}$ |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | AM | MID | PM | Sat |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Enter | 1 | 1 | 1 | 1 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Exit | 1 | 1 | 1 | 1 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Total | 2 | 2 | 2 | 2 |  |
| Lumber Road a |  |  | Soutl | bound |  |  | West | ound |  |  | Nort | bound |  |  | East | bound |  |  |
| Old Northern Boule |  | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | Total |
| Distribution | Entering Exiting |  | 50\% |  | 50\% |  | 50\% |  |  |  |  |  |  |  |  |  | 50\% | $\begin{aligned} & 100 \% \\ & 100 \% \end{aligned}$ |
| Site Generated Volume | AM | --- | 0.5 | --- | 0.5 | --- | 0.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 0.5 | 2 |
| Other Planned Projects | Midday | --- | 0.5 | --- | 0.5 | --- | 0.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 0.5 | 2 |
| 5 Studio Units | PM | --- | 0.5 | --- | 0.5 | --- | 0.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 0.5 | 2 |
|  | SAT | --- | 0.5 | --- | 0.5 | --- | 0.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 0.5 | 2 |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | slyn Lan | ding Phase |  |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  | $\text { Site } \mathbf{S p}$ AM | $\begin{aligned} & \text { cific Tric } \\ & \text { MID } \end{aligned}$ | Generatio PM | Data |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Enter | 16 | 11 | 4 | 4 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Exit | 7 | 18 | 9 | 13 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Total | 23 | 29 | 13 | 17 |  |
| Lumber Road a |  |  | Soutl | bound |  |  | West | ound |  |  | Nort | bound |  |  | East | ound |  |  |
| Old Northern Boule |  | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | Total |
| Distribution | Entering Exiting |  |  |  |  |  |  | 50\% |  |  |  |  |  |  |  | 50\% |  | $\begin{aligned} & 50 \% \\ & 50 \% \end{aligned}$ |
| Site Generated Volume | AM | --- | --- | --- | --- | --- | --- | 3.5 | --- | --- | --- | --- | --- | --- | --- | 8.0 | --- | 12 |
| Other Planned Projects | Midday | --- | --- | --- | --- | --- | --- | 9.0 | --- | --- | --- | --- | --- | --- | --- | 5.5 | --- | 15 |
| Roslyn Landing Phase II | PM | --- | --- | --- | --- | --- | --- | 4.5 | --- | --- | --- | --- | --- | --- | --- | 2.0 | --- | , |
| 50 Units | SAT | --- | --- | --- | --- | --- | --- | 6.5 | --- | --- | --- | --- | --- | --- | --- | 2.0 | --- | 9 |
| Growth Factor: No. of Years: | 1.00\% |  |  |  |  |  |  |  |  |  |  |  |  |  | Propose | Project |  |  |
| No. of Years: |  |  |  |  |  |  |  |  |  |  |  |  |  |  | Trip G | neration D |  |  |
| Growth Rate: | 1.020 |  |  |  |  |  |  |  |  |  |  |  |  | AM | MID | PM | Sat |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Enter | 3 | 7 | 7 | 6 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Exit | 7 | 4 | 5 | 6 |  |
|  |  |  |  |  |  |  |  |  |  |  |  |  | Total | 10 | 11 | 12 | 12 |  |
| Lumber Road a |  |  | Soutl | bound |  |  | West | ound |  |  | Nort | bound |  |  | East | bound |  |  |
| Old Northern Boule |  | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | U-Turn | Right | Through | Left | Total |
| Distribution | Entering |  |  |  |  |  | 50\% |  |  |  |  |  |  |  |  |  | 50\% | 100\% |
|  | Exiting |  | 50\% |  | 50\% |  |  |  |  |  |  |  |  |  |  |  |  | 100\% |
| Site Generated Volume | AM | --- | 3.5 | --- | 3.5 | --- | 1.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 1.5 | 10 |
|  | Midday | --- | 2.0 | --- | 2.0 | --- | 3.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 3.5 | 11 |
|  | PM | --- | 2.5 | --- | 2.5 | --- | 3.5 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 3.5 | 12 |
|  | SAT | --- | 3.0 | --- | 3.0 | --- | 3.0 | --- | --- | --- | --- | --- | --- | --- | --- | --- | 3.0 | 12 |
| Existing AM Peak Hour | 8:00 AM | 0 | 23 | 0 | 14 | 0 | 23 | 427 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 299 | 38 | 824 |
| Existing PM Peak Hour | 12:15 PM | 0 | 79 | 0 | 41 | 0 | 32 | 405 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 621 | 76 | 1254 |
| Existing Midday Peak Hour | 4:00 PM | 0 | 56 | 0 | 21 | 0 | 15 | 329 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 634 | 59 | 1114 |
| Existing Sat Peak Hour | 12:00 PM | 0 | 56 | 0 | 23 | 0 | 18 | 251 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 516 | 45 | 909 |
| AM Adjusted Flow Rate | 0.862 | --- | 27 | 0 | 16 | --- | 27 | 495 | 0 | --- | 0 | 0 | 0 | --- | 0 | 347 | 44 | 956 |
| Midday Adjusted Flow Rate | 0.968 | --- | 82 | 0 | 42 | --- | 33 | 419 | 0 | --- | 0 | 0 | 0 | --- | 0 | 642 | 79 | 1296 |
| PM Adjusted Flow Rate | 0.916 | --- | 61 | 0 | 23 | --- | 16 | 359 | 0 | --- | 0 | 0 | 0 | --- | 0 | 692 | 64 | 1216 |
| Sat Adjusted Flow Rate | 0.924 | --- | 61 | 0 | 25 | --- | 19 | 272 | 0 | --- | 0 | 0 | 0 | --- | 0 | 559 | 49 | 984 |
| Ambient No Build AM | 1.020 | --- | 27 | 0 | 17 | --- | 27 | 505 | 0 | --- | 0 | 0 | 0 | --- | 0 | 354 | 45 | 975 |
| Ambient No Build Midday | 1.020 | --- | 83 | 0 | 43 | --- | 34 | 427 | 0 | --- | 0 | 0 | 0 | --- | 0 | 655 | 80 | 1322 |
| Ambient No Build PM | 1.020 | --- | 62 | 0 | 23 | --- | 17 | 366 | 0 | --- | 0 | 0 | 0 | --- | 0 | 706 | 66 | 1240 |
| Ambient No Build Sat | 1.020 | --- | 62 | 0 | 25 | --- | 20 | 277 | 0 | --- | 0 | 0 | 0 | --- | 0 | 570 | 50 | 1004 |
| No Build AM Peak Hour |  | --- | 28 | 0 | 17 | --- | 28 | 509 | 0 | --- | 0 | 0 | 0 | --- | 0 | 362 | 45 | 989 |
| No Build Midday Peak Hour |  | --- | 84 | 0 | 44 | --- | 34 | 436 | 0 | --- | 0 | 0 | 0 | --- | 0 | 660 | 81 | 1338 |
| No Build PM Peak Hour |  | --- | 63 | 0 | 24 | --- | 17 | 371 | 0 | --- | 0 | 0 | 0 | --- | 0 | 708 | 66 | 1249 |
| No Build Sat Peak Hour |  | --- | 62 | 0 | 26 | --- | 20 | 284 | 0 | --- | 0 | 0 | 0 | --- | 0 | 572 | 50 | 1014 |
| Build AM Peak Hour |  | --- | 31 | 0 | 21 | --- | 29 | 509 | 0 | --- | 0 | 0 | 0 | --- | 0 | 362 | 47 | 999 |
| Build Midday Peak Hour |  | --- | 86 | 0 | 46 | --- | 38 | 436 | 0 | --- | 0 | 0 | 0 | --- | 0 | 660 | 84 | 1349 |
| Build PM Peak Hour |  | --- | 65 | 0 | 26 | --- | 21 | 371 | 0 | --- | 0 | 0 | 0 | --- | 0 | 708 | 70 | 1261 |
| Build Sat Peak Hour |  | --- | 65 | 0 | 29 | --- | 23 | 284 | 0 | --- | 0 | 0 | 0 | --- | 0 | 572 | 53 | 1026 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.1 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\mathbf{A}$ | $\mathbf{F}$ |  | r |  |
| Traffic Vol, veh/h | 44 | 347 | 495 | 27 | 16 | 27 |
| Future Vol, veh/h | 44 | 347 | 495 | 27 | 16 | 27 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 44 | 347 | 495 | 27 | 16 | 27 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 522 | 0 | - | 0 | 944 | 509 |
| Stage 1 | - | - | - | - | 509 | - |
| Stage 2 | - | - | - | - | 435 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1044 | - | - | - | 291 | 564 |
| Stage 1 | - | - | - | - | 604 | - |
| Stage 2 | - | - | - | - | 653 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1044 | - | - | - | 276 | 564 |
| Mov Cap-2 Maneuver | - | - | - | - | 276 | - |
| Stage 1 | - | - | - | - | 573 | - |
| Stage 2 | - | - | - | - | 653 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 1 |  | 0 |  | 14.9 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1044 | - | - | - | 406 |
| HCM Lane V/C Ratio |  | 0.042 | - | - | - | 0.106 |
| HCM Control Delay (s) |  | 8.6 | 0 | - | - | 14.9 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | - | - | 0.4 |



| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: |
| Conflicting Flow All | 537 | 0 | - | 0 | 975 | 523 |
| Stage 1 | - | - | - | - | 523 | - |
| Stage 2 | - | - | - | - | 452 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1031 | - | - | - | 279 | 554 |
| $\quad$ Stage 1 | - | - | - | - | 595 | - |
| Stage 2 | - | - | - | - | 641 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1031 | - | - | - | 264 | 554 |
| Mov Cap-2 Maneuver | - | - | - | - | 264 | - |
| Stage 1 | - | - | - | - | 562 | - |
| Stage 2 | - | - | - | - | 641 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1 | 0 | 15.4 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1031 | - | - | - | 392 |
| HCM Lane V/C Ratio | 0.044 | - | - | -0.115 |  |
| HCM Control Delay (s) | 8.7 | 0 | - | -15.4 |  |
| HCM Lane LOS | A | A | - | - | C |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.4 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.2 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | F |  | Mr |  |
| Traffic Vol, veh/h | 47 | 362 | 509 | 29 | 21 | 31 |
| Future Vol, veh/h | 47 | 362 | 509 | 29 | 21 | 31 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 47 | 362 | 509 | 29 | 21 | 31 |


| Major/Minor | Major1 | Major2 |  |  | Minor2 |  |  |
| :---: | ---: | ---: | ---: | ---: | ---: | ---: | :---: |
| Conflicting Flow All | 538 | 0 | - | 0 | 980 | 524 |  |
| Stage 1 | - | - | - | - | 524 | - |  |
| Stage 2 | - | - | - | - | 456 | - |  |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |  |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |  |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |  |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |  |
| Pot Cap-1 Maneuver | 1030 | - | - | - | 277 | 553 |  |
| $\quad$ Stage 1 | - | - | - | - | 594 | - |  |
| Stage 2 | - | - | - | - | 638 | - |  |
| Platoon blocked, \% |  | - | - | - |  |  |  |
| Mov Cap-1 Maneuver | 1030 | - | - | - | 261 | 553 |  |
| Mov Cap-2 Maneuver | - | - | - | - | 261 | - |  |
| Stage 1 | - | - | - | - | 560 | - |  |
| Stage 2 | - | - | - | - | 638 | - |  |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 1 | 0 | 15.9 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1030 | - | - | - | 381 |
| HCM Lane V/C Ratio | 0.046 | - | - | -0.136 |  |
| HCM Control Delay (s) | 8.7 | 0 | - | -15.9 |  |
| HCM Lane LOS | A | A | - | - | C |
| HCM 95th \%tile Q(veh) | 0.1 | - | - | - | 0.5 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | F |  |
| Traffic Vol, veh/h | 79 | 642 | 419 | 33 | 42 | 82 |
| Future Vol, veh/h | 79 | 642 | 419 | 33 | 42 | 82 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 79 | 642 | 419 | 33 | 42 | 82 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 2.8 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | $\uparrow$ |  | 4 |  |
| Traffic Vol, veh/h | 81 | 660 | 436 | 34 | 44 | 84 |
| Future Vol, veh/h | 81 | 660 | 436 | 34 | 44 | 84 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 81 | 660 | 436 | 34 | 44 | 84 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 470 | 0 | - | 0 | 1275 | 453 |
| Stage 1 | - | - | - | - | 453 | - |
| Stage 2 | - | - | - | - | 822 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1092 | - | - | - | 184 | 607 |
| Stage 1 | - | - | - | - | 640 | - |
| Stage 2 | - | - | - | - | 432 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1092 | - | - | - | 162 | 607 |
| Mov Cap-2 Maneuver | - | - | - | - | 162 | - |
| Stage 1 | - | - | - | - | 565 | - |
| Stage 2 | - | - | - | - | 432 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.9 |  | 0 |  | 24.3 |  |
| HCM LOS |  |  |  |  | C |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1092 | - | - | - | 312 |
| HCM Lane V/C Ratio |  | 0.074 | - | - | - | 0.41 |
| HCM Control Delay (s) |  | 8.6 | 0 | - | - | 24.3 |
| HCM Lane LOS |  | A | A | - | - | C |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | - | - | 1.9 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 3 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | $\uparrow$ |  | 4 |  |
| Traffic Vol, veh/h | 84 | 660 | 436 | 38 | 46 | 86 |
| Future Vol, veh/h | 84 | 660 | 436 | 38 | 46 | 86 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 84 | 660 | 436 | 38 | 46 | 86 |


| Major/Minor | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 474 | 0 | - | 0 | 1283 | 455 |
| Stage 1 | - | - | - | - | 455 | - |
| Stage 2 | - | - | - | - | 828 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1088 | - | - | - | 182 | 605 |
| Stage 1 | - | - | - | - | 639 | - |
| Stage 2 | - | - | - | - | 429 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1088 | - | - | - | 160 | 605 |
| Mov Cap-2 Maneuver | - | - | - | - | 160 | - |
| Stage 1 | - | - | - | - | 561 | - |
| Stage 2 | - | - | - | - | 429 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 1 |  | 0 |  | 25.3 |  |
| HCM LOS |  |  |  |  | D |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT WBR SBLn1 |  |  |
| Capacity (veh/h) |  | 1088 | - | - | - | 307 |
| HCM Lane V/C Ratio |  | 0.077 | - | - | - | 0.43 |
| HCM Control Delay (s) |  | 8.6 | 0 | - | - | 25.3 |
| HCM Lane LOS |  | A | A | - | - | D |
| HCM 95th \%tile Q(veh) |  | 0.2 | - | - |  | 2.1 |


| Intersection |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Int Delay, s/veh 1.6 |  |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | $\uparrow$ |  | M |  |
| Traffic Vol, veh/h | 64 | 692 | 359 | 16 | 23 | 61 |
| Future Vol, veh/h | 64 | 692 | 359 | 16 | 23 | 61 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | \# | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 64 | 692 | 359 | 16 | 23 | 61 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: |
| Conflicting Flow All | 375 | 0 | - | 0 | 1187 | 367 |
| Stage 1 | - | - | - | - | 367 | - |
| Stage 2 | - | - | - | - | 820 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1183 | - | - | - | 208 | 678 |
| $\quad$ Stage 1 | - | - | - | - | 701 | - |
| Stage 2 | - | - | - | - | 433 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1183 | - | - | - | 190 | 678 |
| Mov Cap-2 Maneuver | - | - | - | - | 190 | - |
| Stage 1 | - | - | - | - | 639 | - |
| Stage 2 | - | - | - | - | 433 | - |


| Approach | EB | WB | SB |
| :--- | :---: | :---: | :---: |
| HCM Control Delay, s | 0.7 | 0 | 16.4 |
| HCM LOS |  |  | C |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |  |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1183 | - | - | - | 398 |
| HCM Lane V/C Ratio | 0.054 | - | - | -0.211 |  |
| HCM Control Delay (s) | 8.2 | 0 | - | -16.4 |  |
| HCM Lane LOS | A | A | - | - | C |
| HCM 95th \%tile Q(veh) | 0.2 | - | - | - | 0.8 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | F |  | Mr |  |
| Traffic Vol, veh/h | 66 | 708 | 371 | 17 | 24 | 63 |
| Future Vol, veh/h | 66 | 708 | 371 | 17 | 24 | 63 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 66 | 708 | 371 | 17 | 24 | 63 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.7 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | 1 |  | Mr |  |
| Traffic Vol, veh/h | 70 | 708 | 371 | 21 | 26 | 65 |
| Future Vol, veh/h | 70 | 708 | 371 | 21 | 26 | 65 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 70 | 708 | 371 | 21 | 26 | 65 |



| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | $\uparrow$ | F |  | Mr |  |
| Traffic Vol, veh/h | 49 | 559 | 272 | 19 | 25 | 61 |
| Future Vol, veh/h | 49 | 559 | 272 | 19 | 25 | 61 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 49 | 559 | 272 | 19 | 25 | 61 |


| Major/Minor | Major1 | Major2 |  | Minor2 |  |  |
| :--- | ---: | :--- | :--- | :--- | ---: | ---: |
| Conflicting Flow All | 291 | 0 | - | 0 | 939 | 282 |
| Stage 1 | - | - | - | - | 282 | - |
| Stage 2 | - | - | - | - | 657 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | -3.518 | 3.318 |  |
| Pot Cap-1 Maneuver | 1271 | - | - | - | 293 | 757 |
| $\quad$ Stage 1 | - | - | - | - | 766 | - |
| Stage 2 | - | - | - | - | 516 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1271 | - | - | - | 277 | 757 |
| Mov Cap-2 Maneuver | - | - | - | - | 277 | - |
| Stage 1 | - | - | - | - | 723 | - |
| Stage 2 | - | - | - | - | 516 | - |


| Approach | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| HCM Control Delay, s | 0.6 | 0 | 13.6 |
| HCM LOS |  |  | B |


| Minor Lane/Major Mvmt | EBL | EBT | WBT | WBR SBLn1 |
| :--- | ---: | ---: | ---: | ---: |
| Capacity (veh/h) | 1271 | - | - | -503 |
| HCM Lane V/C Ratio | 0.039 | - | - | -0.171 |
| HCM Control Delay (s) | 7.9 | 0 | - | -13.6 |
| HCM Lane LOS | A | A | - | - |
| HCM $95 t h$ \%tile Q(veh) | 0.1 | - | - | - |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.6 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | -1 | 1 |  | Mr |  |
| Traffic Vol, veh/h | 50 | 572 | 284 | 20 | 26 | 62 |
| Future Vol, veh/h | 50 | 572 | 284 | 20 | 26 | 62 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, $\%$ | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 50 | 572 | 284 | 20 | 26 | 62 |


| Major/Minor M | Major1 |  | Major2 |  | Minor2 |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Conflicting Flow All | 304 | 0 | - | 0 | 966 | 294 |
| Stage 1 | - | - | - | - | 294 | - |
| Stage 2 | - | - | - | - | 672 | - |
| Critical Hdwy | 4.12 | - | - | - | 6.42 | 6.22 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.42 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.42 | - |
| Follow-up Hdwy | 2.218 | - | - | - | 3.518 | 3.318 |
| Pot Cap-1 Maneuver | 1257 | - | - | - | 282 | 745 |
| Stage 1 | - | - | - | - | 756 | - |
| Stage 2 | - | - | - | - | 508 | - |
| Platoon blocked, \% |  | - | - | - |  |  |
| Mov Cap-1 Maneuver | 1257 | - | - | - | 266 | 745 |
| Mov Cap-2 Maneuver | - | - | - | - | 266 | - |
| Stage 1 | - | - | - | - | 712 | - |
| Stage 2 | - | - | - | - | 508 | - |
|  |  |  |  |  |  |  |
| Approach | EB |  | WB |  | SB |  |
| HCM Control Delay, s | 0.6 |  | 0 |  | 14 |  |
| HCM LOS |  |  |  |  | B |  |
|  |  |  |  |  |  |  |
| Minor Lane/Major Mvmt |  | EBL | EBT | WBT | WBR SBLn1 |  |
| Capacity (veh/h) |  | 1257 | - | - | - | 486 |
| HCM Lane V/C Ratio |  | 0.04 | - | - | - | 0.181 |
| HCM Control Delay (s) |  | 8 | 0 | - | - | 14 |
| HCM Lane LOS |  | A | A | - | - | B |
| HCM 95th \%tile Q(veh) |  | 0.1 | - | - | - | 0.7 |


| Intersection |  |  |  |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: |
| Int Delay, s/veh | 1.8 |  |  |  |  |  |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations |  | - | T |  | M |  |
| Traffic Vol, veh/h | 53 | 572 | 284 | 23 | 29 | 65 |
| Future Vol, veh/h | 53 | 572 | 284 | 23 | 29 | 65 |
| Conflicting Peds, \#/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage, \# | - | 0 | 0 | - | 0 | - |
| Grade, \% | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 100 | 100 | 100 | 100 | 100 | 100 |
| Heavy Vehicles, \% | 2 | 2 | 2 | 2 | 2 | 2 |
| Mvmt Flow | 53 | 572 | 284 | 23 | 29 | 65 |




[^0]:    ${ }^{1}$ It is a violation of New York State Education Law Section 7209.2 for any person, unless acting under the direction of a licensed professional engineer, to alter these documents in any way. If altered, the altering engineer shall affix to these documents his seal and the notation "altered by" followed by his signature and the date of such alteration, and a specific description of the alteration.

